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GEOTECHNICAL AND HYDROGEOLOGIC INVESTIGATION Proposed Landfill Site for Dickson County Dickson County, Tennessee



Dickson County Sanitary Landfill 100 Virgil Bellar Drive Dickson, Tennessee 37055

Prepared By:

ATEC Associates, Inc. 624 Grassmere Park Road Suite 1 Nashville, Tennessee 37211

May 13, 1992



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Mr. Virgil Bellar Dickson County Sanitary Landfill 100 Virgil Bellar Drive Dickson, Tennessee 37055

RE: Geotechnical and Hydrogeologic Investigation

Proposed Landfill Site Dickson County, Tennessee

ATEC File Number: 24-03-92-00010

Dear Mr. Bellar:

In compliance with your recent request, we have completed a geotechnical and hydrogeologic investigation for the above referenced project. It is our pleasure to transmit herewith two (2) copies of our written report of the results of this investigation.

This report contains the results of our findings and an engineering interpretation of these findings with respect to the available project characteristics. We will store the samples for 60 days after which time they will be discarded unless you request otherwise.

If you should have any questions concerning this or any other matter, please feel free to contact us at your convenience. It has been a pleasure working with you on this project.

Respectfully Submitted.

ATEC ASSOCIATES, INC.

L. Greg Stephenson, R.G.

Project Geologist

Timothy G. LaGrow, P.E. SP

Tennessee District Manager

LGS/TGL/da Enclosure

cc: Mr. Bob Gardner (2)

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GEOTECHNICAL AND HYDROGEOLOGIC INVESTIGATION PROPOSED LANDFILL SITE FOR DICKSON COUNTY, TENNESSEE ATEC FILE NUMBER 24-3-92-00010

1.0 EXECUTIVE SUMMARY

An exploration and evaluation of the subsurface conditions have been conducted at the site of a proposed landfill for Dickson County, Tennessee. The purpose of the investigation was to determine the depth and types of subsoils present throughout the site, to evaluate the suitability of these soils for use as landfill liner and buffer materials, and to provide the required Hydrogeologic Report outlined in the Tennessee Department of Health and Environment (TDHE), Division of Solid Waste Regulations Chapter 1200-1-7-.04(9)(a).

Six (6) exploratory borings have been performed throughout a 35 acre site shown on the enclosed Boring Location Plan. The results of these borings have identified one (1) distinct soil stratum consisting of red, moderate to highly plastic silty clay with varying amounts of chert. This stratum extends to the parent bedrock at depths of 64 feet or greater. The results of our investigation indicate that this stratum is suitable for use as the landfill buffer zone. Laboratory testing indicates that the permeability of the stratum is typically $1.0x10^{-6}$ centimeters per second (cm/s) or lower.

Hydrogeologic information available for this area indicates that the uppermost aquifer occurs within the Warsaw limestone, approximately 20 to 50 feet below the bedrock surface at this site. The three (3) existing wells at the landfill are suitable to monitor the water moving through the overburden and recharging the underlying aquifer within the bedrock. Additional monitoring points should include the small tributary to Worley Furnace Branch immediately west of the site, and either the two existing private wells south of the landfill or Baker Branch.

2.0 INTRODUCTION

This report presents the results of a geotechnical investigation and hydrogeologic evaluation of the proposed sanitary landfill site in Dickson County, Tennessee. This study was performed for the client, Dickson County Sanitary Landfill, in accordance with our signed proposal dated October 22, 1991.

The scope of this investigation included a review of available geologic data for the project area, an engineering reconnaissance of the project location, a subsurface investigation consisting of six (6) soil test borings located as shown on the attached Boring Location Plan, field and laboratory soil testing, and an engineering analysis and evaluation of the subsurface materials encountered.

The purpose of the investigation was to determine the types of subsoils present throughout the site, to evaluate the suitability of these soils for use as landfill liner and buffer materials, and to provide the required Hydrogeologic Report outlined in the Tennessee Department of Health and Environment (TDHE), Division of Solid Waste Regulations Chapter 1200-1-7-.04(9)(a).

3.0 SITE AND PROJECT CHARACTERISTICS

The proposed landfill site is located on a 40 acre tract of land on the north side of Eno Road, just east of Adcock Road, in Dickson County. The site is approximately 1.5 miles east of Dickson, Tennessee and 4.5 miles north of Interstate 40. Figure 1 in Appendix A depicts the location of the property. The site is bordered to the north and west by wooded areas, Worley Furnace Branch and it's tributaries. The property to the east is also partially wooded and partially developed.

Five (5) acres of the property on the east side of the site are currently being used as a landfill under the old regulations. This section of the landfill will be closed while the remaining 35 acres will be permitted under the new regulations. Current operations consist of excavating approximately 20 feet of overburden and placing bailed refuse above the natural undisturbed soil. It is expected that the newly permitted landfill design will require excavation of the residual overburden to a predetermined depth on which to construct a landfill liner system in accordance with Tennessee Division of Waste Management regulations. Conversations with the client and Mr. Bob Gardner (project designer) indicate that a synthetic liner will be used to meet the minimum liner design standard outlined in Section 1200-1-7-.04(4)(a)

The topography of the 35 acres proposed as landfill is partially wooded. This property can best be described as gently rolling. A topographic survey of the property is included on the Boring Location Plan (Figure 2) in Appendix A.

Boring No.	1	3	5	6
Depth (ft)	25.0-26.5	30.0-32.0	18.5-20.0	15.0-15.6
Density (pcf)	109.6	97.3	104.0	118.8
Moisture Content (%)	18.3	25.4	19.1	11.9
Rate of Permeability (cm/sec)	3.4x10 ⁻⁷	6.5x10 ⁻⁸	1.5x10 ⁻⁶	1.5x10 ⁻⁷

All borings were extended to a depth of auger refusal between 23.0 to 45.0 feet below the present ground surface. Based on past drilling at the site, we believe that refusal occurred on dense chert beds within the soil. Limestone bedrock is expected to be approximately 65 to 90 feet below the present ground surface.

Published geologic information indicates that the underlying bedrock is the St. Louis Limestone of the Mississippian Period. This bedrock consists of a pale to dark yellowish brown, fine to very coarse grained, thin to very thick bedded limestone. Local beds of extremely cherty, olive gray, calcareous siltstone or very silty limestone exist, especially near the base of the formation. Bedrock exposures are rare in this area. In most cases, the bedrock has weathered to a thick stratum of reddish clay soil and angular blocks of chert, similar to that described in our borings.

It should be noted that the St. Louis Limestone is a formation highly susceptible to dissolution along joints and bedding planes in the rock mass. This weathering can result in soil piping and other forms of internal soil erosion which can result in ground subsidence. These features are not prominent in the Dickson area. No sinkholes were observed on the Dickson 7.5 minute quadrangle map within a 1.5 mile radius of the site.

5.0 HYDROGEOLOGIC ASSESSMENT

Four monitoring wells currently exist at the site, one of which has been dry since installation. The location of the three (3) active wells is shown on the Boring Location Plan. The construction of the three active wells is reported to consist of 4 inch PVC pipe, screened from 10 to 20 feet above the bedrock surface. The screened interval is surrounded by pea gravel to approximately 5 feet above the top of the screen with a 2 to 5 foot thick bentonite seal above the gravel. The remainder of the riser pipe is surrounded by cement.

Groundwater elevation readings taken on March 16, 1992 revealed the following data.

WELL	SURFACE ELEV.	BOTTOM WELL EL.	DEPTH TO WATER (ft)	WATER ELEV.
1	861.9	772.9	78.0	783.9
2	830.9	766.4	19.2	811.7
. 4	831.2	747.2	35.2	796.0

The results of these readings indicate that the groundwater being monitored by these wells is perched within the overburden immediately above the bedrock surface. The placement of the screened interval within the relatively impermeable clays immediately above the bedrock will allow for the groundwater to mound at the well points. This has resulted in an undefinable groundwater flow direction in the overburden using these three wells.

A review of the report "Groundwater in the Dickson Area of the Western Highland Rim of Tennessee", published in 1984, reveals that the groundwater in this area occurs primarily in the Warsaw Limestone, the top of which is near Elev. 720 to 750. Figure 3 in Appendix A shows the location of 20 known wells within a 1 mile radius

Materials excavated from the site should be adequate for use as temporary and final cover material. Final confirmation should be made on remolded permeabilities of representative samples taken from the borrow source. At this time, we expect that the soil removed during excavation will be used. The soil should be compacted to at least 95 percent of the standard Proctor maximum dry density within a moisture range of zero (0) to three (3) percent above optimum. Final recommendations can be made after testing of the soil.

Our review of the available hydrogeologic information indicates that the existing well placement at the site intersects the groundwater recharging the underlying aquifer in the Warsaw limestone. Based on the site topography, Well No. 1 is up gradient and Well Nos. 2 and 4 are down gradient. These wells have a good chance of intersecting any contaminated groundwater moving through the overburden before it reaches the aquifer. To properly use these wells, it will be necessary to bail them on a monthly basis before sampling to insure proper recharge and groundwater movement to the wells because of mounding effects.

In addition to the three (3) existing monitoring well points, it is recommended that monitoring occur in the small tributary to Worley Furnace Branch immediately west of the site, and the private wells owned by J. Pickett and R. Buchannan south of the site. If these wells are unavailable to monitor, then a monitoring point should be set up at Baker Branch.

APPENDIX A

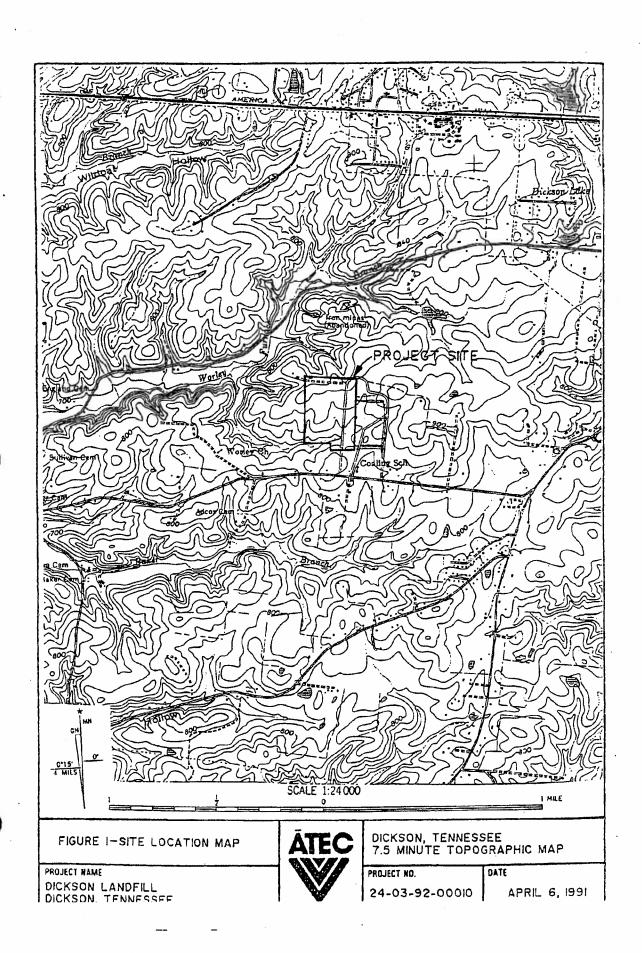
Figure 1 - Site Location Map

Figure 2 - Boring Location Plan

Figure 3 - Water Well Location Map

Figure 4 - Potentiometric Contour Map of Dickson Are

Table No. 1 - Water Well Location Information



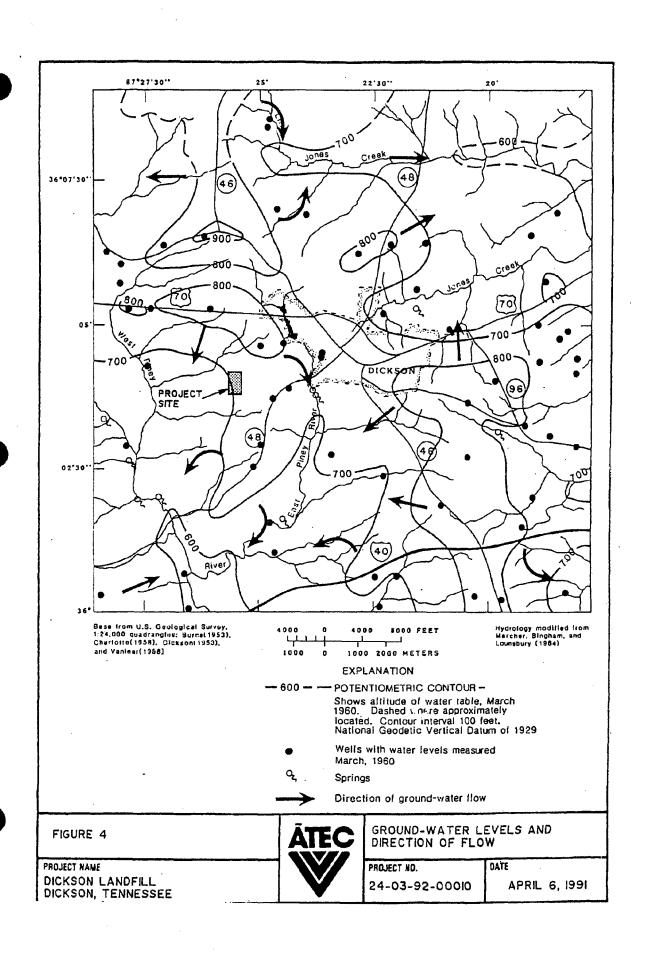


TABLE NO. 1 DICKSON COUNTY SANITARY LANDFILL WATER WELL INFORMATION

WATER WELL DATE TOTAL BOTTOM BEARING TOTAL NO. ZONE OWNER COMPLETED YIELD USE DEPTH CASING 277 C. Bradford 4/18/67 100 87 Home 5 358 J. Puckett 7/12/68 79 Home 160 3 220 521 J. Holt 3/25/64 129 4 128 125 Home 525 R. Holt 7/16/70 300 2 98 260 Home 528 A. Harris 8/31/70 360 71 120 Home 552 G. Donegan 2/24/70 105 50 100 100 Home 663 R. Buchanan 7/30/71 127 130 10 130 828 E. Lovelace 4/28/73 200 2 47 Home 851 J. Horner 6/18/73 160 5 Home 852 H. Holt 6/25/73 340 160 Home 1384 City of Dickson 300 12/1/80 4 220 136 Municipal 1385 City of Dickson 10/20/80 400 160 106 143 Municipal 1386 City of Dickson 20 10/6/80 250 106 116 Municipal 1387 City of Dickson 10/4/80 300 8 103 252 Municipal 1388 City of Dickson 10/2/80 250 150 181 197 Municipal 1389 City of Dickson 8/4/80 300 150 144 180 Municipal 1390 7/24/80 City of Dickson 350 115 307 14 Municipal 1392 City of Dickson 7/14/81 280 165 127 145 Municipal 1393 City of Dickson 320 245 12 162 Municipal 1396 City of Dickson 7/7/80 280 110 127 130 Municipal 1397 City of Dickson 7/2/80 340 175 318 330 Municipal 9029 K. Walker 75 75 Home 9030 J. Robinson 212 107 Home 9040 D. Sanders 100 110 Home 9041 D. Sanders 138 Home 9046 D. Donegan 125 Home

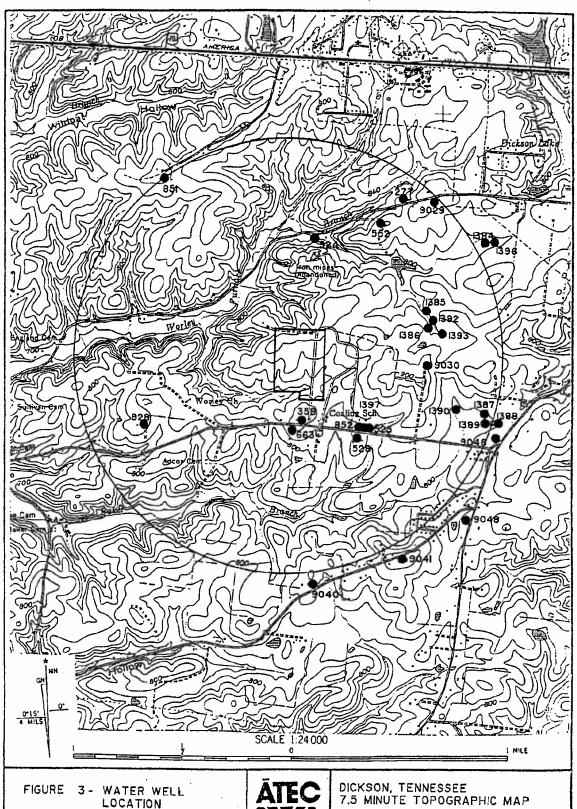
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Home

9048

W. R Street



LOCATION

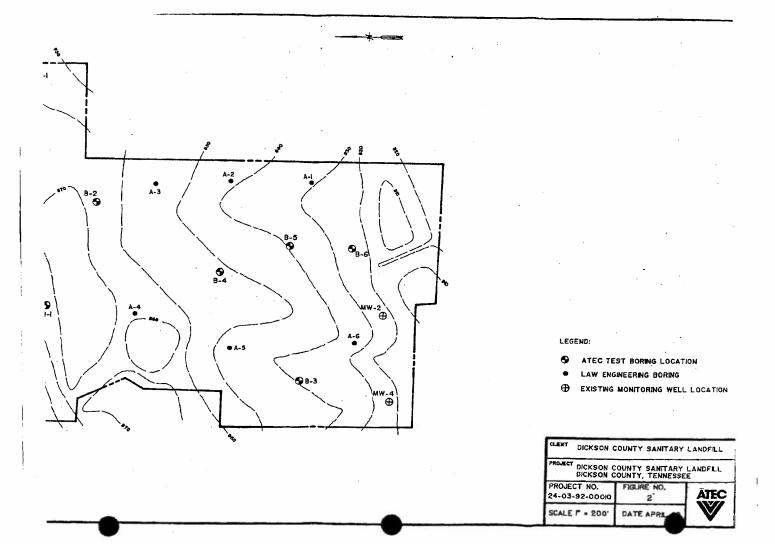
PROJECT NAME DICKSON LANDFILL DICKSON, TENNESSEE



PROJECT NO.

24-03-92-00010

APRIL 6, 1991

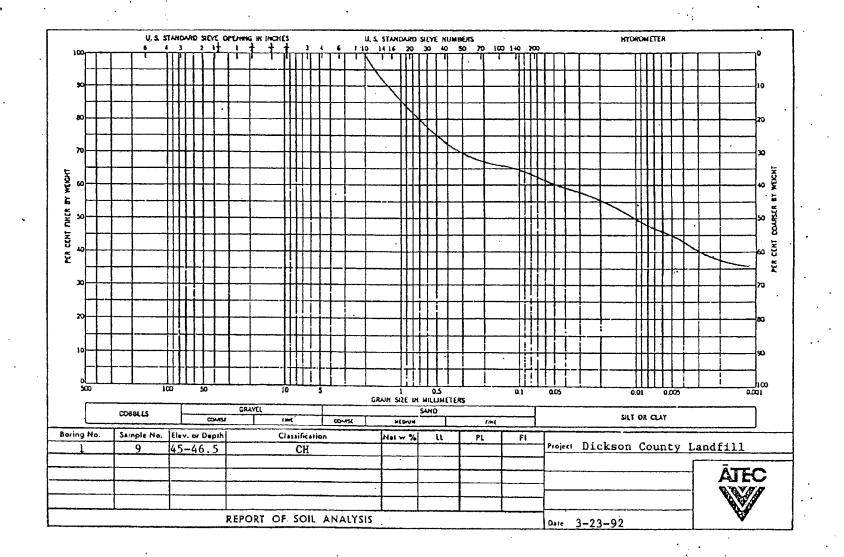


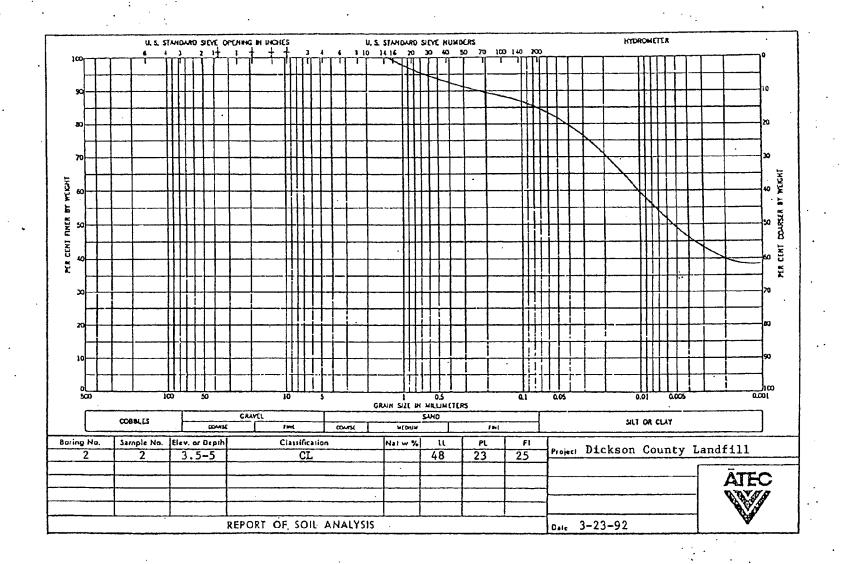
DICKSON COUNTY SANITARY LANDFILL LABORATORY TEST RESULTS SUMMARY

			SAMPLE		ATTER	BERG L	<u>IMITS</u>			
BORING	SAMPLE	DEPTH	TYPE	CLASS.	<u>LL</u>	<u>PL</u>	<u>PI</u>	STEVE	HYDROM	PERM. (cm/sec)
1	1	6.0-6.6	55							
	2	10-11.5	83	CL.	34	21	13		•	
	3	15-16.5	88							
	4	20-21.5	3 3	CL.	37	17	20			
	5	25-26.5	S S							3.4E-07
	6	30-31.5	S S	GP .				•		
	7	35-36.5	S S							
	8	40-41.5	CS	CH	54	22	42			
	9	45-48.5	39						•	
	10	50-51.5	33	•						•
2	1	1.0-2.5	53							•
	2	3.5-6:0	58	OL.	48	23	25		•	_
	3	4.0-7.5	53	GP				•		
	4	8.6-10	88					- ,		•
	5	13.5-15	32	CL	35	15	20		•	
	6	18.5-20	98							
	7	23.5-25	S S	GP				•		
	•	28.5~30	S S							
	0	33.5 -35	SS							
3	1	15-16.5	88	GP				•		
	2	20-21.5	CS							
	3	25-26.5	88					•		6.5E-08
	4 5	30–32	CS 99	СН		~				0.32-06
	6	35-38.5 40-41.5	55 58	CA	58	21	35	•		
	7	45-46.5	55 85							
	,	10-10.0	23							
4	1	5.0-5.5	88	GP						
	2	10-11.6	88	СН	56	24	32			
	3	15-18.5	C9	•						
	4	20-21.5	68							
	5	25-26.5	53							
	6	30-31.5	39	Q.P				•		
	7	35-36.5	95	***						
		40-41.5	39	CL.	47	18	29			

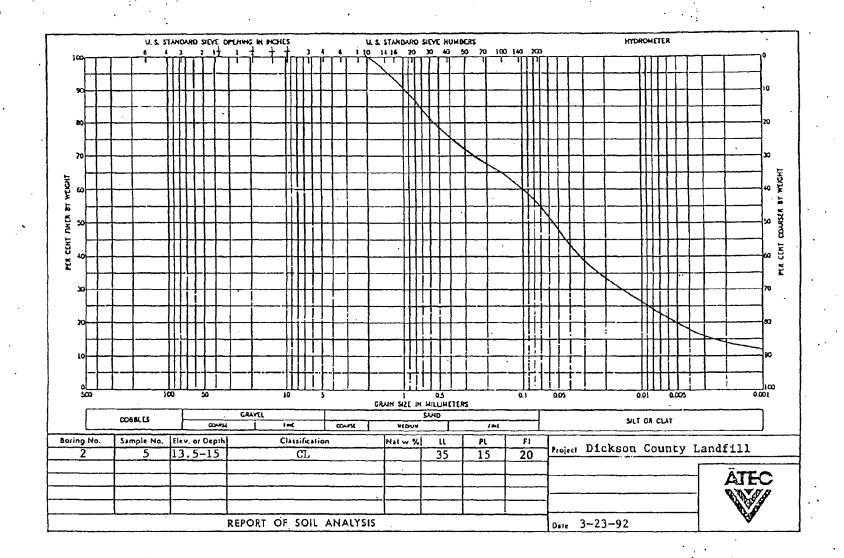
			SAMPLE		ATTER	BERG L	IMITS			
BORING	SAMPLE	DEPTH	TYPE	CLASS.	<u>LL</u>	PL	PI	STEVE	HYDROM	PERM.
5	1	6.0-6.5	83	GP				•		
	2	10-11.6	53							
	3	13-15	ST							
	4	20-22	ST							1.5E-06
	5	25-27	CS							
	6	35-34.5	SS							
	7	40-42	st							
					,					
	1.1	15-16.6	CS							1.5E-07
	2	20-21.5	88						•	
	3	25-26.5	98 .	7				•		
	4	30-31.5	83	CL.	41	21	20			
	5	35~36.6	89	GP				•		
	8	4041.5	88							
		AVERAGE			42	17	24			

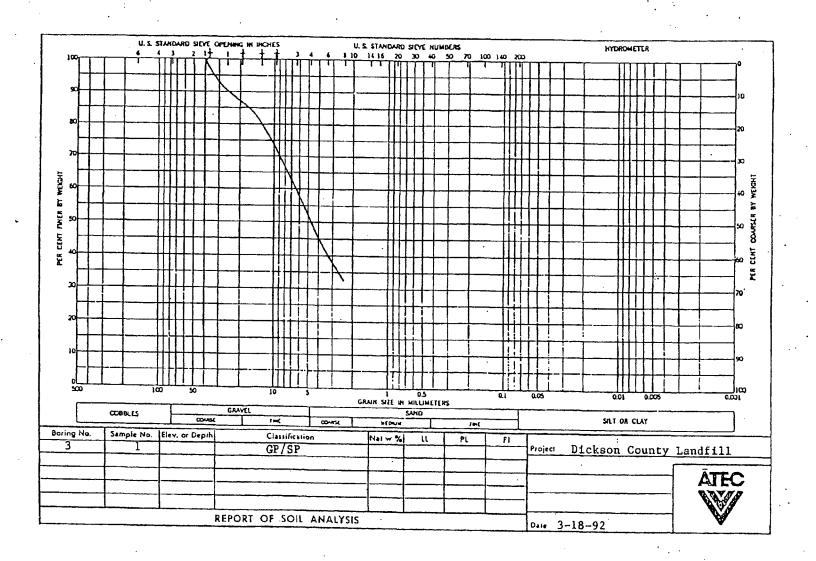
^{*} See enclosed graph sheet

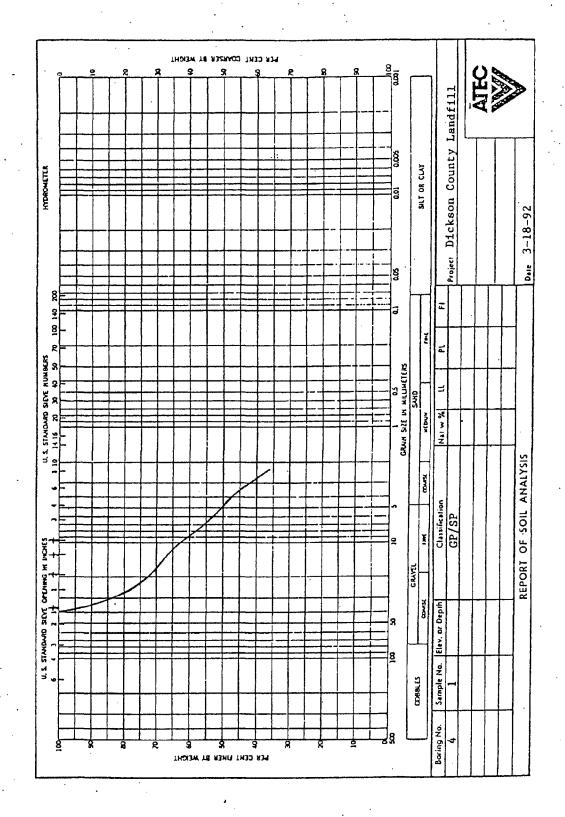




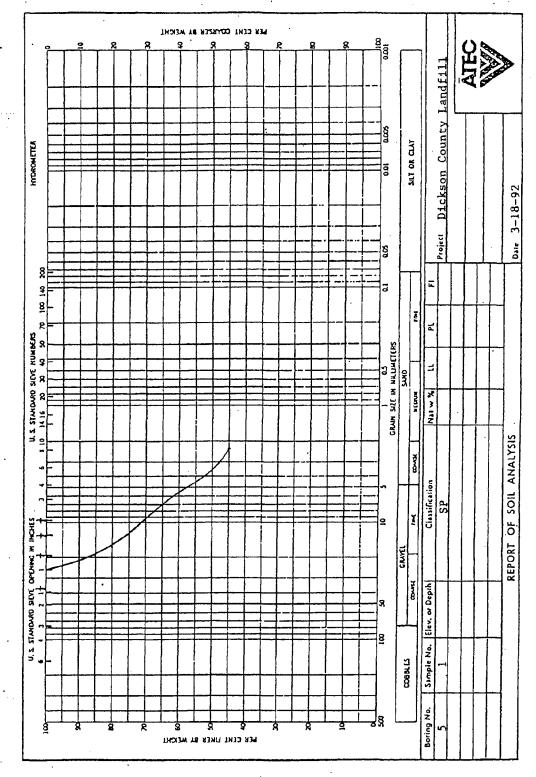
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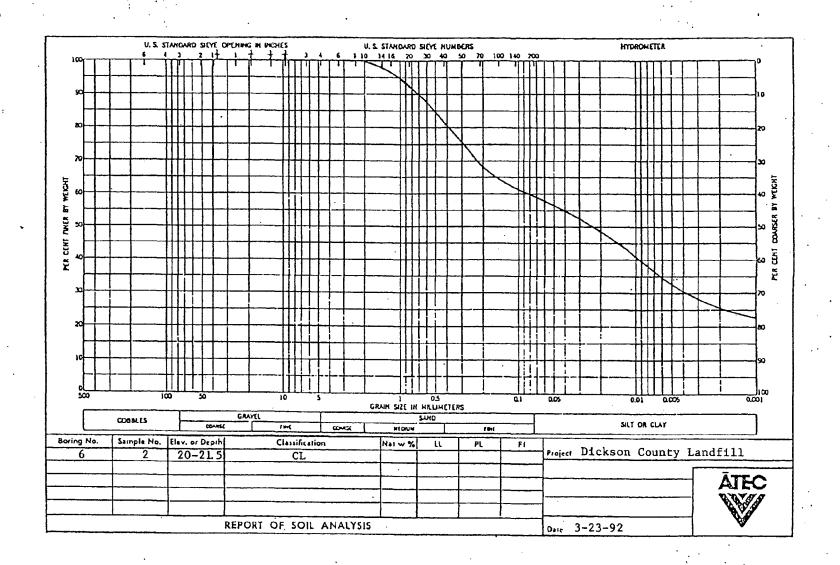


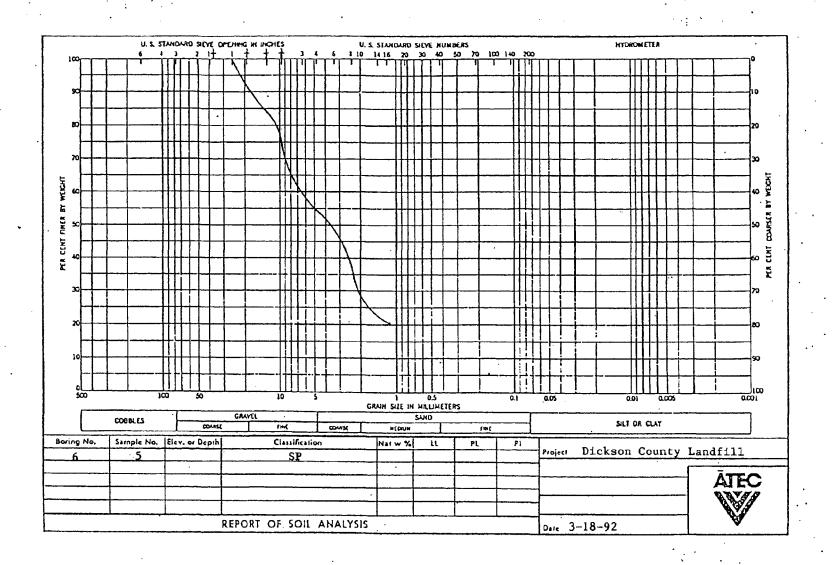


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APPENDIX C

Logs of Boring

Field Classification System for Soil Exploration

Important Information About Your Geotechnical Engineering Report



LABORATORY REPORT OF SOIL EXPLORATION

Client	Dickson County Sanitar	y Land	£111					Boring #		B-1	Page 2 c	of 2	
Architect Engineer_				_							2-00010		
Project Name	Dickson County Sanitar	y Land	£111										
Project Location	Dickson, Tennessee							Approved	Ву	TL	~		
									TE	ST DATA			
	NG AND SAMPLING INFORMA												,
Date Started0	3/03/92 Hammer Wt	14	الــــــــــــــــــــــــــــــــــــ	9.				tş:		15			İ
Date Completed 0	3/03/92 Hammer Drop) "	SS	_			ļ
Unit Foreman	Spoon Sampler O.						~	욜	8d	e e	}	İ	
	S Rock Core Dia.				Ψ	≥	Œ	l gtu	Compressive	g	Ë	=	2 2 2
Boring Method -H				n.	₹ .	VER	Š	ية يز	ع = يت	Fi.*	نجح	nee	52.5
SOIL CL	ASSIFICATION	ĮξΞ	ΞЩ	P.E.	SAMPLE TYPE	RECOVERY	GROUND WATER	dard	ons.	et P	ral C	Water Content	rberg Units Liquid Limit Plastic Limit
SURFACE	E ELEVATION —	STRATUM DEPTH	DEPTH	SAMPLE NO.	SAM	% H	GRO	Standard Penetration Test N, Blows/Ft.	Unconfined C Strength Q.v Tons/Fl.	Pockal Penetrometer q.p Tons/F1.*	Natural Dry Density Ibs./cu. ft.	Wate %	Atterberg Units LL - Liquid Limil PL - Plastic Lim
		29.0	"	1									
	-	123.0	30	1									ĺ
Clay (CH) and che	ist stiff to hard silty ert gravel with trace		30-	!	1								1
fine sand		}	=	6	SS	55		17			ŀ		ļ
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some small chert	below 35.0 feet	<u>l</u>	35-	1									ł
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		}	-	-									
		}	40_] 	cs	33							
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		51.5)	11	ss	100		21					
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Boring discontinu	ed at 51.5 feet			}									
		1] =	1	1			-					
		}	=	1									
			_	1									
SAMPLER	TYPE	GR	OUND	WATE	R DE	TH		L	L	BOF	ING ME	HOD	L
SS-DRIVEN SPLIT S ST-PRESSED SHELE	SPOON VA	COM	PLETIO	и			_ F	т.		-HOLLOV	V STEM A	UGERS	CEDE
A-CONTINUOUS F	LIGHT AUGER	FTER _							DC-	DRIVING		IGHI AU	GENS
C-ROCK CORE	·	ATER C	וטא אינ	۔۔۔ در			F	i.		MUD DRI			



LABORATORY REPORT OF SOIL EXPLORATION

Projec	ct Location	DICKSON,	Tennessee							Approve	d ByTL	ST DATA			
Date Drill insp	Started	02/1 <u>4/92</u> 02/20/92 DV JS	MPLING INFORM/ Hammer Wt Hammer Drop Spoon Sampler O. Rock Core Dia Shelby Tube O.D.	146 30 D	2	in. in. in.	YPE	RY	NATER	Standard Penetration Test N, Blows/Ft.	Unconfined Compressive Strength q.u Tons/Ft.*	etrometer 1.2	/ Density I.	tent	Jufts Limit c Limit
		LASSIFICA E ELEVAT		STRATUM DEPTH	DEPTH	SAMPLE NO.	SAMPLE TYPE	% RECOVERY	GROUND WATER	Standard F	Jnconfined Strength J.u Tons/F	Pocket Penetrometer q.p Tons/Ft.²	Natural Dry Density Ibs./cu. ft.	Water Content	Alterberg Units LL - Liquid Limi PL - Plastic Lim
Bro cla	soil wn moist sti y (CL) with ce small wea	trace fine	y stiff silty e sand and ert.	5		1	ss	100		13		-	-	-	
				5.5	5 ~	2	SS	100		19					LL=48 PL=23
cla	y with trace	fine sand		8.0		3	SS	100		49			•		
(CL	nge to red m) with trace thered chert	fine sand	silty clay and some		10-	4	SS	70		43				-	
					15—	5	SS	70		_. 50+					LL=35 PL=15
Ve	ry moist at	20.0 feet			20-	6	SS	100		50+					
	athered cher ay with trac		GP) and silty	22.5 _26.0	25—	7	ss	100		41					
Or. wi	ange to red th trace fin gravel	moist silt	y clay (CL) i some chert							·				•	



LABORATORY REPORT OF SOIL EXPLORATION

Client	Dickson County Sanit	ry La	ndfill			· · ·		Boring #		B-3	Page 2	of 2	
Architect Engineer	Gardner Engineering Dickson County Sanita	y La	ndfill				_	Job # Drawn B:		.TO	92-00010		
Project Name	Dickson, Tennessee							Approved	y I By	TL			
, , , , , , , , , , , , , , , , , , , ,									•	ST DATA			
. DOUGLING	S AND SAMPLING INFORM	ATION							16	DAIA			
	4/92 Hammer Wt.		140 %								1		١.
	4/92 Hammer Drop							Test	2				
Drill Foreman DV								5	ess	5	_	ļ	
Inspector JS	Rock Core Dia						#	irat	E G	ê E	ns.		==
Boring MethodCF.	A Shelby Tube O.D.		i	n.	YPE	EH	₹	نب تو	S -	orte:	å	lent	를들
	SSIFICATION			ш	LET	Ş	Q	Brd F Ws/F	liner ngth ns/Fi	Pan Is/Fi	. D.	Conf	astic
SUBFACE	ELEVATION —	STRATUM	DEPTH	SAMPLE	SAMPLE TYPE	% RECOVERY	GROUND WATER	Standard Penetration Test N, Blows/Ft.	Unconfined Compressive Strength q.v. Tons/Ft.	Pocket Panetrometer q.p Tons/Ft.²	Natural Dry Densily Ibs./cu. fl.	Water Content	ă i
JOHNAGE		SO	29	ώz	S	126	9	űΖ	2 4	ጀ ዓ	ž	_ }	316
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4	. (011)		_	1								·	
chert	clay (CH) with some	}	25 =	1									
Very moist at 35.0) feet		35	十 <u>-</u>				~ -					
	*		_	5	SS	100		36					
			_	1		į							
-			_	1		1							
7			_	1									
Moist, orange belo	w 40.0 feet	}	40.			1							
7			=	6	SS	100		38					
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1		46.5	 	7	SS	100		50+	 				
Boring discontinue	d at 46.5 feet		=	1 1									
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SAMPLER TY	/PF	C D	OUND	WATE	B DE		اا		L!				
SS-DRIVEN SPLIT SP	OON ∇A	COME	LETIO	N			_ F	т.	HSA-	-HOLLOY	ING MET V STEM A	UGERS	
ST—PRESSED SHELBY CA—CONTINUOUS FLI	CUT ARCED VA	FTER _					_ F	T.	CFA-	CONTIN	UOUS FL	IGHT AU	GERS'
RC-ROCK CORE	. W	ATER C	OR N	S			F	т.		MUD DRI			