

Title:	GEOLOGIC LOGGING
Category:	GEO 4.8
Revised:	March 1998

GEOLOGIC LOGGING

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1. Introduction

Geologic logging involves keeping detailed records during the drilling of boreholes, the installation of monitoring wells, and the excavation of test pits, and entering the geologic descriptions of the soil and rock samples recovered on a standardized form. E & E has adapted a standardized geotechnical logbook (see DOC 2.4 in E & E's Standard Operating Procedures [SOPs]) that contains items deemed important to record when installing monitoring wells, piezometers, and/or soil borings. This document discusses general procedures for completing geologic logs.

2. Drilling Logs

2.1 Basic Documentation

When drilling boreholes, the project geologist should maintain a log that describes each borehole. The E & E Geotechnical Logbook contains records for boreholes. The following basic information should be entered on the heading of each drilling log sheet (see Figure 1):

- Borehole/well number;
- Project name;
- Site location;
- Dates and times that drilling was started and completed;
- Drilling company;
- E & E geologist's name;
- Drill rig type used to drill the borehole;
- Drilling method(s) used to drill the borehole;



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	ect Name					Water Level (TOIC)					
Site	Site Location					Date		Time		Level (Fee	et)
Date	Started/F	inished									
Drilli	ng Compa	iny									
Geo Rig T Drilli Bit S Aug	ologist's Na ologist's Sig Type(s) — ing Method Size(s) — er/Split Sp	gnature ————————————————————————————————————	_ Auger Size(s)			Well Los	cation Sket				•
Tota	Sample Number		Soil Components CL SI S GR	Rock Pile	Penetration Times	Run Number	Core Recovery	RQD	Fracture Sketch	HNu/OVA (ppm)	Commen
			Components					RQD			Commen

Figure 1 Drilling Log





- Bit and auger size(s);
- Depth of auger/split barrel sampler refusal;
- Total depth of borehole;
- Total depth of corehole (if applicable);
- Water level at time of completion measured from top of inside casing (TOIC); and
- A well location sketch.

2.2 Technical Information

During the drilling of a borehole, specific technical information about the unconsolidated material and rock encountered should be recorded on the drilling log sheet. The following minimum technical information should be recorded:

- Depth that sample was collected or encountered;
- Sample number assigned (if applicable);
- The number of blow counts required to drive the split barrel sampler 2 feet at 6-inch intervals (see Table 1);
- Description of soil components (see Figure 2);
- Description of rock profile (see Figure 3);
- Rock qualitative designation (RQD) (see Figure 4);
- Rock penetration time;
- Core run number (if applicable) and percent recovery; and
- Organic vapor readings in the sample.



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Table 1 Standard Penetration Test for Soil Density

3011 Density						
N-Blows/Feet	Relative Density					
Cohesionless Soils						
0 - 4	Very loose					
4 - 10	Loose					
10 - 30	Medium					
30 - 50	Dense					
50	Very dense					
Cohesive Soils						
2	Very soft					
2 - 4	Soft					
4 - 8	Medium					
8 - 15	Stiff					
15 - 30	Very stiff					
30	Hard					

3. Soil Classification

Soils should be described using the Unified Soil Classification System (USCS) in the narrative lithologic description section of Figure 5. Figure 6 is a summary of the American Society for Testing and Materials (ASTM) criteria for describing soils. Soil descriptions should be concise, stressing major constituents and characteristics, and should be given in a consistent order and format. The following order is recommended by the ASTM:

- 1. Soil name. The basic name of the predominant constituent and a single-word modifier indicating the major subordinate constituent.
- 2. Gradation or Plasticity. Granular soils (i.e., sands or gravels) should be described as well-graded, poorly-graded, uniform, or gap-graded, depending on the gradation of the minus 3-inch fraction. Cohesive soils (i.e., silts and clays) should be described as nonplastic, slightly plastic, moderately plastic, or highly plastic, depending on results of the manual evaluation for plasticity.
- 3. Particle size distribution. An estimate of the percentage and grain-size range of each subordinate constituent of the soil. This description may also include a description of angularity (see Figure 7).
- 4. Color. The basic color of the soil.





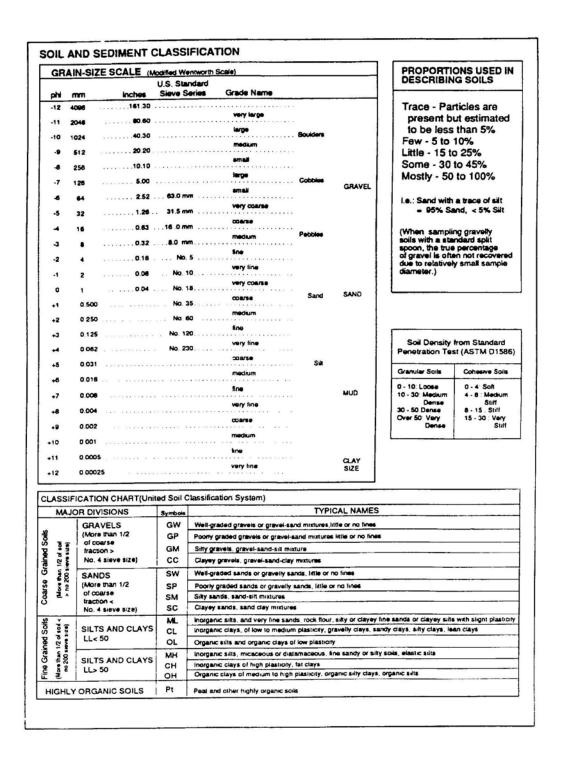


Figure 2 USCS Soil Classification Chart





	Term	Definir	ng Characteristics			
Hardness	Soft Moderately Hard Hard Very Hard	Scratched by fingernail Scratched easily by penknife Difficult to scratch with a penknife Cannot be scratched by penknife				
Weathering	Unweathered	Rock is unstained. May be fractured, but discontinuities are not stained.				
	Slighty	Rock is unstained. Discontinuities show some staining on the surfaces of rocks, but discoloration does not penetrate rock mass.				
	Moderate	Discontinuity surfaces a	are stained. Discoloration may extend into			
	High	rock along discontinuity	v surfaces.			
Individual rock fragments are thoroughly stained and crumbly.						
	Severe		st of gravel-sized fragments in a "soil" matrix e thoroughly discolored and can be broken			
Bedding	Laminated	< .04 ln.	< 1 mm			
Planes	Parting	.04 in24 In.	1mm - 6mm	1		
	Banded	.24 in 1in.	6 mm - 3 cm			
	Thin	1 in 4 in.	3 cm - 9.1 cm			
	Medium Thick	4 in 12 in. 12 in 36 in.	9.1 cm - 30.5 cm 30.5 cm - 1 m			
	Massive	> 36 in.	> 1 m			
Joints and	Very tight	< 2 in.	< 5.1 cm	_		
Fracture	Tight	2 in 1ft.	5.1 - 30.5 cm			
Spacing	Moderately tight	1ft 3 ft.	30.5 cm - 91.4 cm			
	Wide	3 ft 10 ft.	91.4 cm - 3 M			
	Very wide	> 10 ft.	> 3 M			
Volds	Porous	Smaller than a pinhead of absorbency.	i. Their presence is indicated by the degree			
	Pitted		nch. If only thin walls separate the individual escribed as honeycombed.			
	Vug	1/4 inch to the diamete core size.	r of the core. The upper limit will vary with			
	Cavity	Larger than the diamet	er of the core.			
ock Particle Per	cent Composition Estimation	on				
(;·						
(· ::)				,		
1%	F 0/	10% 159	% 25% 50%			

Figure 3 Rock Descriptive Terms



ROCK QUALITY DESIGNATION (RQD) AND FRACTURE FREQUENCY

Core borings are a useful means of obtaining information about the quality of rock mass. The recoverable core indicates the character of the intact rock and the number and character of the natural department of the continuities.

Another quantitative index that has proved useful in logging NX core is a rock quality designation (RQD) developed by Deere (1963). The RQD is a modified core recovery percentage in which all the pieces of sound NX core over 4 inches long are counted as recovery. The length of the core run is the distance to the nearest tenth of a foot from the corrected depth of the hole at the end of the previous run to the corrected depth of the hole at the end of subject run. The smaller pleose are considered to be due to close shearing, jointing, faulting, or weathering in the rock mass and are not counted. The RQD is a more general measure of the core quality than the fracture frequency. Core loss, weathered and soft zones, as well as fractures, are accounted for in this determination. The RQD provides a preliminary estimate of the variation of the In situ rock mass properties from the properties of the "sound" portion of the rock core. Thus, a general estimate of the behavior of the rock mass can be made. An RQD approaching 100 percent denotes an excellent quality rock mass with properties similar to that of an intact specimen. RQD values ranging from 0 to 50 percent are indicative of a poor quality rock mass having a small fraction of the strength and stiffness measured for an intact specimen.

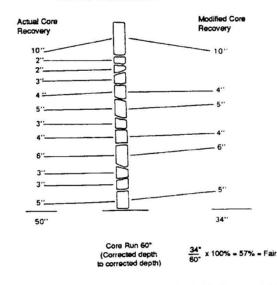
RQD (Rock Quality Designation)

0 - 25 Very Poor 25 - 50 Poor 50 - 75 Fair 75 - 90 Good 90 - 100 Excellent An example of determining the RQD from a core run of 60 inches measured from corrected depth to corrected depth is given in Diagram 1. For this particular case, the core recovery was 50 inches and the modified core recovery was 34 inches. This yields an RQD of 57 percent, classifying the rock mass in the fair category.

Problems arise in the use of RQD for determining the in situ rock mass quality. The RQD evaluates fractures in the core caused by the drilling process, as well as in natural fractures previously existing in the rock mass. For example, when the core hole penetrates a fault zone or a joint, additional breaks may form that, although not natural fractures, are caused by natural planes of weakness existing in the rock mass. These fresh breaks occur during drilling and handling of the core and are not related to the quality of the rock mass. The skill of the driller will affect the amount of breakage and the core loss that occurs. Poor drilling bechniques will "penalize" the rock by lowering its apparent quality. It is difficult to distinguish between drilling breaks and those natural and incipient fractures that reflect the quality of the rock mass. In certain instances, it may be advisable to include all fractures when estimating RQD. Obviously, some judgement is involved in core logging.

Another problem with the use of the RQD index is that the determinations are not sensative to the lightness of the individual joints, whereas in some instances, the in situ deformation modulus may be strongly affected by the average joint opening.

RQD OF A SINGLE CORE RUN*

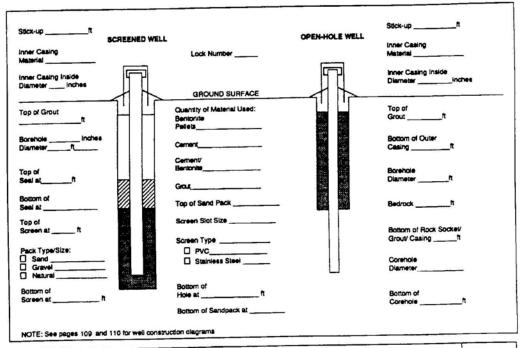


^{*} Typical calculation of RQD of a single core run. Note that the run is calculated from corrected depth to corrected depth.

Figure 4 Rock Qualitative Designation (RQD)







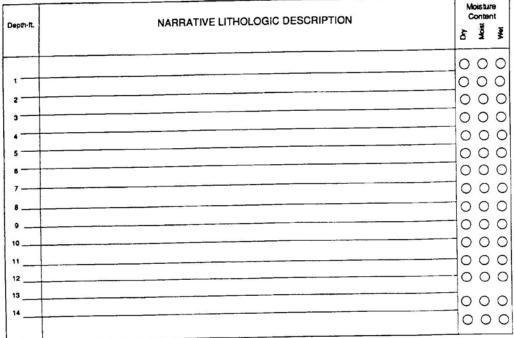


Figure 5 Narrative Lithologic Description





Criteria for I	Describing Angularity of Coarse-	Criteria for I	Describing Dry Strength		
Description	Criteria	Description	Criteria		
Angular	Particles have sharp edges and relatively plane side with unpolished surfaces	None	The dry specimen crumbles into powder with mere pressure of handling		
Subangular	Particles are similar to angular description but have rounded	Low	The dry specimen crumbles into powder with some finger pressure		
Subrounded	edges Particles have nearly plane sides but have well-rounded comers and	Medium	The dry specimen breaks into pieces or crumbles with considerable finger pressure		
Rounded	edges Particles have smoothly curved side and no edges	High	The dry specimen cannot be broken with finger pressure. Specimen will break into pieces between thumb and a hard surface.		
Criteria for I	Describing Dilatancy	Very High	The dry specimen cannot be broken between the thumb and shard surface		
Description	Criteria		55.1400		
None	No visible change in the specimen.	Criteria for I	Describing Structure		
Slow	Water appears slowly on the surface of the	Description Criteria			
	specimen during shaking and does not disappear or disappears slowly upon squeezing.	Stratified	Alternating layers of varing material or color with layers at least 6 mm thick; note thickness.		
Rapid	Water appears quickly on the surface of the specimen during shaking and disappears quickly upon squeezing.	Laminated	Alternating layers of varying materials or color with the layers less than 6 mm thick; note thickness.		
Criteria for [Describing Toughness	Fissured	Breaks along definite planes of fracture with little resistance to fracturing.		
Description	Criteria	Slickensided	Fracture planes appear polished		
Low	Only slight pressure is required to roll the thread near the plastic limit.	Silcherisided	or glossy, sometimes striated.		
	The thread and the lump are weak and soft.	Blocky	Cohesive soil that can be broken down into small angular lumps which resist further breakdown.		
Medium	Medium pressure is required to roll the thread to near plastic limit. The thread and the lump have medium stiffness.	Lensed	Inclusion of small pockets of different soils, such as small lenses of sand scattered through a mass of clay; note thickness.		
High	Considerable pressure is required to roll the thread to near the plastic limit. The thread and the lump have very high stiffness.	Homo- geneous	Same color and appearance throughout.		

Figure 6 ASTM Criteria For Describing Soil





Criteria for I	Describing the Reaction with HCI	Criteria fe	or Describing 1	Plasticity			
Description	Criteria	Description	on Criteria				
None	No visible reaction	Nonplastic A 1/8 inch (3 mm) the rolled at any water or					
Weak	Some reaction, with bubbles forming slowly	Low	The thread	The thread can barely be rolled and the lump cannot be formed when drier than the plastic limit.			
Strong	Violent reaction, with bubbles forming immediately	Medium	drier than				
Criteria for	Criteria for Describing Consistency Description Criteria		much time	The thread is easy to roll and not much time is required to reach the plastic limit. The thread cannot be rolled after reaching the plastic limit. The lump crumbles when drier than			
Description			The lump				
Very Soft	Thumb will penetrate soil more than 1 inch (25 mm)	High	the plastic		me rolling and		
Soft	Thumb will penetrate soil about 1 inch (25 mm)	-	kneading to The thread times after	It takes considerable time rolling and kneading to reach the plastic limit. The thread can be rerolled several times after reaching the plastic limit			
Firm	Thumb will indent soil about 1/4 inch (6 mm)	ž		The lump can be formed without crumbling when drier than the plastic			
Hard	Thumb will not indent soil but readily indented with thumbnail		ation of inorga	anic Fine-G	rained Soils		
Very Hard	Thumbnail will not indent soil	Soil		Dilatana	Tauahaaa		
Criteria for	Describing Cementation	Symbol	Dry Strength	Dilatancy	Toughness		
Description	Criteria	ML	None to low	Slow to rapid	Low or thread cannot be formed		
Weak	Crumbles or breaks with handling or little finger pressure	CL	Medium to high	None to slow	Medium		
Moderate	Crumbles or breaks with considerable finger pressure	МН	Low to	None to	Low to medium		
Strong	Will not crumble or break with finger pressure	СН	High to very	None	High		
Criteria for	Describing Particle Shape		high				
	shape shall be described as follows , width, and thickness refer to ermediate, and least dimensions of a	Criteria 1	or Describing	Moisture Co	ondition		
		Description	on Criteria				
greatest, inte	ectively (see page 104).						
greatest, inte particle, resp		Dry	Absence of the touch	of moisture, o	dusty, dry to		
greatest, inte	pectively (see page 104). Particles with width/thickness	Dry Moist	the touch	of moisture, o			

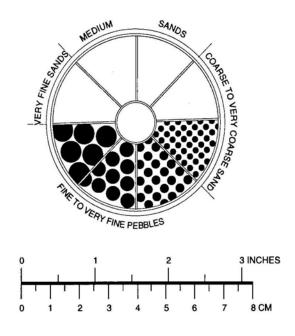
Figure 6 ASTM Criteria for Describing Soil (cont.)





SEDIMENT PARTICLE SIZE AND SHAPE ESTIMATES

GRAPH FOR DETERMINING SIZE OF SEDIMENTARY PARTICLES



COBBLES RANGE FROM 6.4 TO 25.6 cm (~2.5 TO 10.1 INCHES) BOULDERS ARE LARGER THAN 25.6 cm (>10.1 INCHES)

SEDIMENT PARTICLE SHAPES

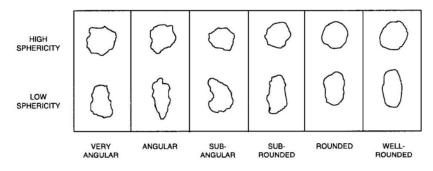


Figure 7 Sediment Particle Size and Shape Estimates





- 5. Moisture content. The amount of soil moisture (dry, moist, or wet).
- 6. Relative density or consistency. An estimate of density of a granular soil or consistency of a cohesive soil, usually based on the standard penetration test results (see Table 1).
- 7. Soil Structure or Mineralogy. Description of discontinuities, inclusions, and structures. Includes joints, fissures, and slickensides.

4. Core Logging

4.1 Handling of Core

After the core has been recovered from the corehole and the core barrel has been opened, the core should be placed in a core box. The top of the core should be placed at the back left corner of the core box, and the remaining core placed to the right of the preceding section (see Figure 8). The core box should be filled in this manner, moving to the front sections of the core box. The beginning of each run should be marked on the core and also noted with a marked wooden block.

4.2 Rock Description

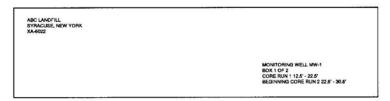
Each stratigraphic unit in the core shall be logged. A line marking the depth of the top and the bottom of the unit shall be drawn horizontally. In classifying the rock, the geologist should avoid being too technical, as the information presented must be used by numerous people with widely divergent backgrounds.

The classification and description of each unit should be given in the following order, as applicable:

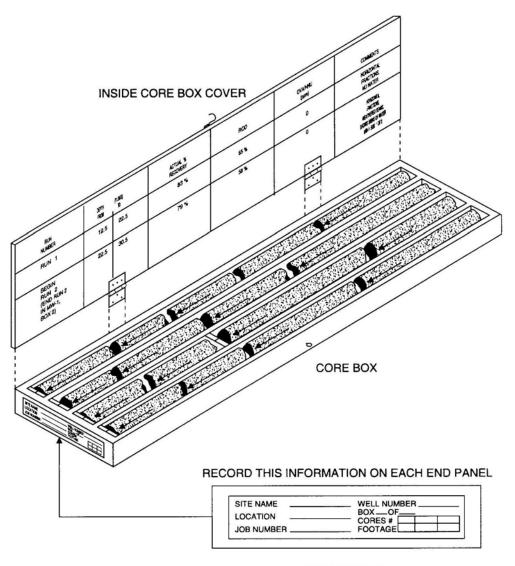
- 1. Unit designation (Miami oolite, Clayton Formation, Chattanooga shale);
- 2. Rock type;
- 3. Hardness;
- 4. Degree of weathering;
- 5. Texture;
- 6. Structure;







EXAMPLE: OUTSIDE CORE BOX COVER



SIDE PANELS

Figure 8 Core Box





- 7. Color;
- 8. Solution and void conditions;
- 9. Swelling properties;
- 10. Slaking properties; and
- 11. Additional description, such as mineralization, size, and spacing shale seams, etc.

Variations from the general description of the unit and features not included in the general description shall be indicated by brackets and lines to show the depth and the interval in the core where the feature exists. These variations and features shall be identified by terms that will adequately describe the feature or variation so as to delineate it from the unit. These may be zones or seams of different color, texture, etc., from that of the unit as a whole, such as staining; variations in texture; shale seams, gypsum seams, chert nodules, calcite masses, etc.; mineralized zones; vuggy zones, joints, fractures; open and/or stained bedding planes; faults, shear zones, gouge; cavities' thickness, open or filled, nature of filling, etc.; or any core left in the bottom of the hole after the final pull.

Rock Type and Lithology

- 1. Rock will be classified according to the following 24 types:
 - Sandstone
 - Conglomerate
 - Coal
 - Compaction Shale
 - Cemented Shale
 - Indurated Clay
 - Limestone
 - Chalk
 - Gneiss
 - Schist





- Graywacke
- Quartzite
- Dolomite
- Marble
- Soapstone and Serpentine
- Slate
- Granite
- Diorite
- Gabbro
- Rhyolite
- Andesite
- Basalt
- Tuff or Tuff Breccia
- Agglomerate or Flow Breccia
- 2. Lithologic characteristics should be included to differentiate rocks of the same classification. These adjectives should be simple and easily understood, such as shaly, sandy, dolomitic, etc. Inclusions, nodules, and concretions should also be noted here.
- 3. It is important to maintain a simple but accurate rock classification. The rock type and lithologic characteristics are essentially used to differentiate the rock units encountered.

Hardness

The terms for hardness, as outlined below, were modified to include the use of a rock hammer.

1. **Very soft** or plastic - can be deformed by hand (has a rock-like character but can be broken easily by hand).



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- 2. **Soft** can be scratched with a fingernail (cannot be crumbled between fingers but can be easily pitted with light blows of a geology hammer).
- 3. **Moderately hard** can be scratched easily with a knife; cannot be scratched with a fingernail (can be pitted with moderate blows of a geology hammer).
- 4. **Hard** difficult to scratch with a knife (cannot be pitted with a geology hammer but can be chipped with moderate blows of the hammer).
- 5. **Very hard** cannot be scratched with a knife (chips can be broken off only with heavy blows of the geology hammer).

Weathering

The degree and depth of weathering is very important and should be accurately detailed in the general description and clearly indicated on the drill log.

- 1. **Unweathered** no evidence of any mechanical or chemical alteration.
- 2. **Slightly weathered** superficial discoloration, alteration, and/or discoloration along discontinuities; less than 10% of the rock volume is altered; strength is essentially unaffected.
- 3. **Moderately weathered** discoloration is evident; surface is pitted and altered, with alterations penetrating well below rock surfaces; 10% to 50% of the rock is altered; strength is noticeably less than unweathered rock.
- 4. **Highly weathered** entire section is discolored; alteration is greater than 50%; some areas of slightly weathered rock are present; some minerals are leached away; retains only a fraction of its original strength (wet strength is usually lower than dry strength).
- 5. **Decomposed** saprolite; rock is essentially reduced to a soil with a relic rock texture; can be molded or crumbled by hand.

Texture

Texture is used to denote the size of the grains or crystals comprising the rock, as opposed to the arrangement of the grains or crystals, which is considered a structure.

1. **Aphanitic** - grain diameter less than 0.004 inch (0.1 mm); individual grains or crystals are too small to be seen with the naked eye.



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- 2. **Fine-grained, finely crystalline** grain diameter between 0.004 inch (0.1 mm) and 0.003 (1 mm); grains or crystals can be seen with the naked eye.
- 3. **Medium-grained, crystalline** grain diameters between 0.003 foot (1 mm) and 0.0175 foot (5 mm).
- 4. **Coarse-grained, coarsely crystalline** grain diameter greater than 0.0175 foot (5 mm).

Structure

The structural character of the rock shall be described in terms of grain or crystal alignment, bedding, and discontinuities, as applicable. The terms may be used singularly or paired.

1. **Foliation and/or lineation** - give approximate dip uniformity, degree of distinctiveness, banding, etc.

2. **Joints:**

- a. Type bedding, cleavage, foliation, extension, etc.
- b. Degree of openness tight or open.
- c. Surface or joint plane characteristics smooth, rough, undulating.
- d. Weathering degree, staining.
- e. Frequency see (4).

3. Fractures, shears, gouge:

- a. Nature single plane or zone. (Note thickness.)
- b. Character of materials in plane or zone.
- c. Slickensides.

4. Frequency:

- a. Intact spacing greater than 6 feet (2 m).
- b. Slightly jointed (fractured) spacing 3 feet (1 m) to 6 feet (2 m).
- c. Moderately jointed (fractured) spacing 1 foot (0.3 m) to 3 feet (1 m).
- d. Highly jointed (fractured) spacing 0.3 foot (9.1 cm) to 1 foot (0.3 m).
- e. Intensely jointed (fractured) spacing less than 0.3 foot (9.1 cm).
- 5. **Bedding** is used to describe the average thickness of the individual beds within recognized unit, and the terms thick, medium, or thin should not be applied to the individual beds. "Parting" and "band" are used to describe single stratum as outlined below:
 - a. Massive over 3 feet thick (1 m).
 - b. Thick 1 foot (30.5 cm) to 3 feet (1 m) thick.
 - c. Medium 0.3 foot (9.1 cm) to 1 foot (30.5 cm) thick.
 - d. Thin 0.1 foot (3.0 cm) to 0.3 foot (9.1 cm) thick.





- e. Band 0.02 foot (6 mm) to 0.1 foot (3.0 cm) thick, described to the nearest 0.01 foot.
- f. Parting less than 0.02 foot (6 mm).
- g. Paper-thin parting.

The terms and descriptions for the structure of the rock are to be used to describe the character of the rock units recognized and are not to be used as a substitute for describing individual discontinuities. Except for areas where the rock is intensely fractured or jointed, each discontinuity should be described on the log as to position, dip, staining, weathering, breccia, gouge, etc.

Color is often valuable in correlating or differentiating samples, but can be misleading or uninformative. The color of a sample should represent the sample in terms of basic hues (i.e., red, blue, gray, black), supplemented with modifying hues as required (i.e., bluish gray, mottled brown). The core should be surface wet when describing the color; if it is dry, the log should indicate "dry color." Subjective colors, such as buff or maroon, should not be used. Specific color charts, such as the Mumsel Color Chart or the Color Index in the Colorado School of Mines, Quarterly, Volume 50, No. 1, are useful in describing color of samples. When such a chart or index is used, it should be noted on the log in the remarks column.

Solution and Void Conditions shall be described in detail, as these features can affect the strength of the rock and can indicate potential seepage paths through the rock. When cavities are detected by drill action, the depth to top and bottom of the cavity should be determined by measuring the stick-up of the drill tools when the cavity is first encountered and again at the bottom, as it is very difficult to reconstruct cavities from the core alone. Filling material, when present and recovered, should be described in detail opposite the cavity. When no material is recovered from the area of the cavity, the inspector should note the probable conditions of the cavity as determined from observing the drilling action and the color of the drill fluid. If the drill action indicated material was present (i.e., slow rod drop, no loss of drill water, noticeable change in color of water return), it should be noted on the log that the cavity was probably filled and the materials should be described as best as possible from the cuttings or traces left on the core. If drill action indicates the cavity was open (i.e., no resistance to the drill tools, loss of drill fluid), this should be noted on the drill log. Partially filled cavities should also be noted. All of these observations require close observation of the drill action and water return by both the inspector and the driller; accurate measurement of stick-ups; and detailed inspection of the core. When possible, filling material should be wrapped in foil if left in the core box. If the material is to be tested or examined in the lab, it should be sealed in a jar with proper labels and a spacer, with a note showing the disposition of the material should be placed in the core box at the point from which the material was taken. Terms to describe voids encountered shall be as follows:

- 1. **Porous** voids less than 0.003 foot (1 mm) in diameter.
- 2. **Pitted** voids 0.03 foot (1 mm) to 0.02 foot (6 mm) in diameter.
- 3. **Vug** voids 0.02 foot (6 mm) to the diameter of the core.
- 4. **Cavity** voids greater than diameter of the core.





4.3 Core Labeling

The top of the core should be shown on each piece of core with an arrow written in a black, waterproof marker. The arrow will indicate which end of the core is nearer the ground surface. Other core markings may include locations of mechanical breaks and drilling footages.

4.4 Core Box Labeling

Each core box should be labeled as follows:

- On the top left corner of the outer core box, the project name, site location (city and state), and project number should be written.
- On the lower right corner of the outer core box, the corehole number (e.g., MW1, BH2), core box number (e.g., 1 of 2, 2 of 2), and the interval of the core run contained in the core box should be written.
- The side panels should be marked as indicated in Figure 8.
- The inside of the core box cover should be marked as indicated in Figure 8.

4.5 Core Storage

It is important to use proper-sized (HQ or NQ) wooden core boxes for rock core storage. After labeling the box and before closing the box for final storage or shipment, wooden spacers should be inserted into each compartment that contains rock core. This will prevent lateral movement of the cores, which could damage the rock material during handling.

After properly logging, labelling, and packing the cores, the core boxes should be stored in a dry location, preferably off of the floor on a pallet. The boxes can be stacked to a reasonable height so as not to be unstable, with end labelling facing out.

5. References

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